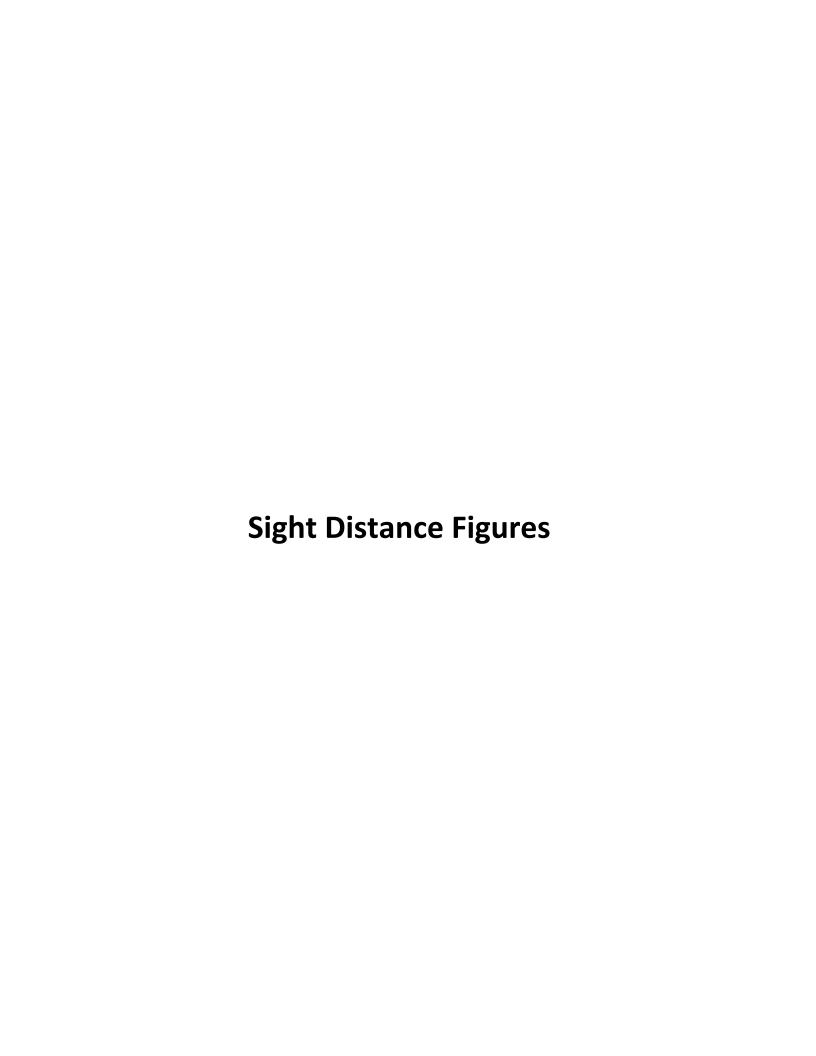
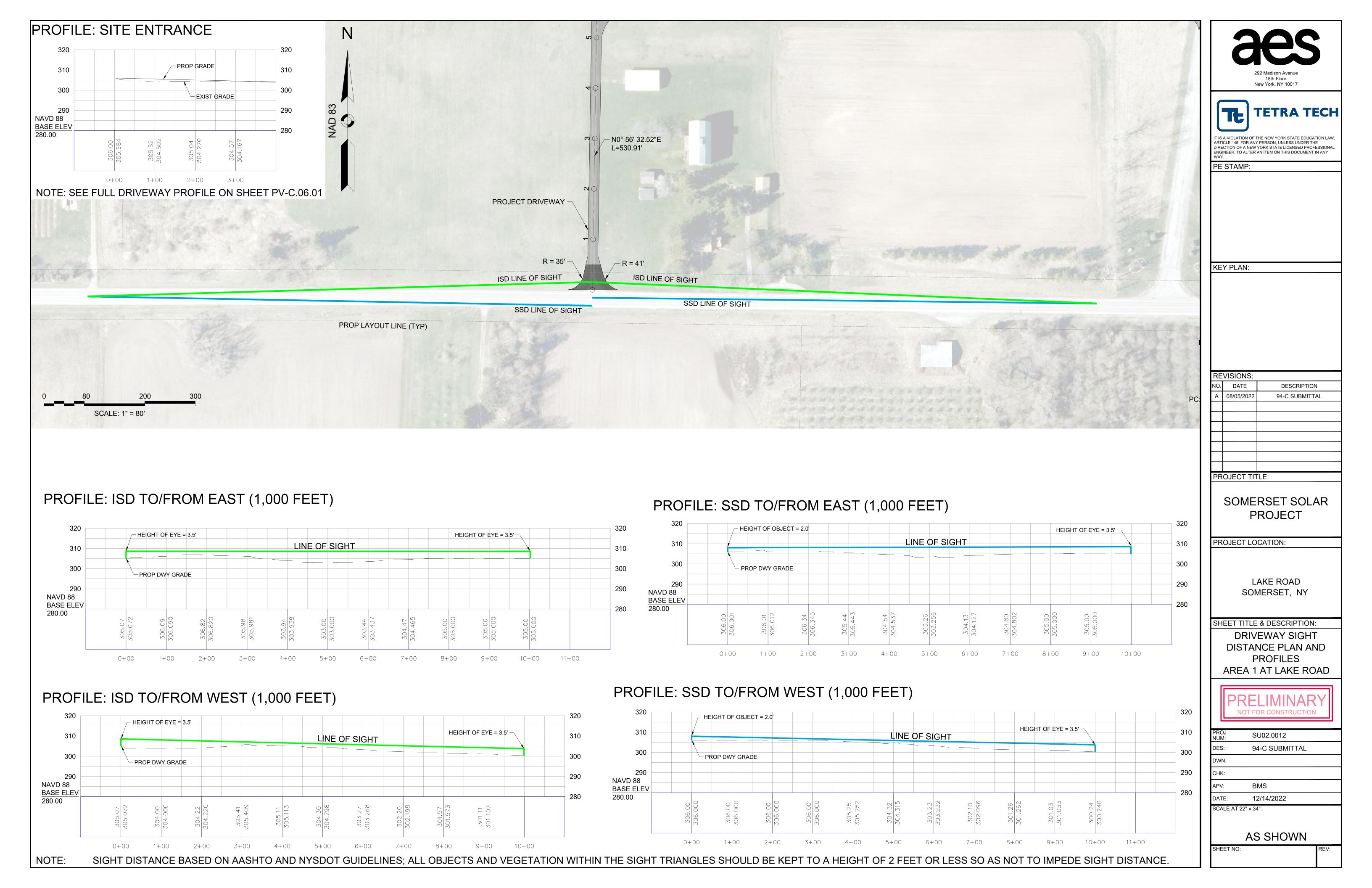


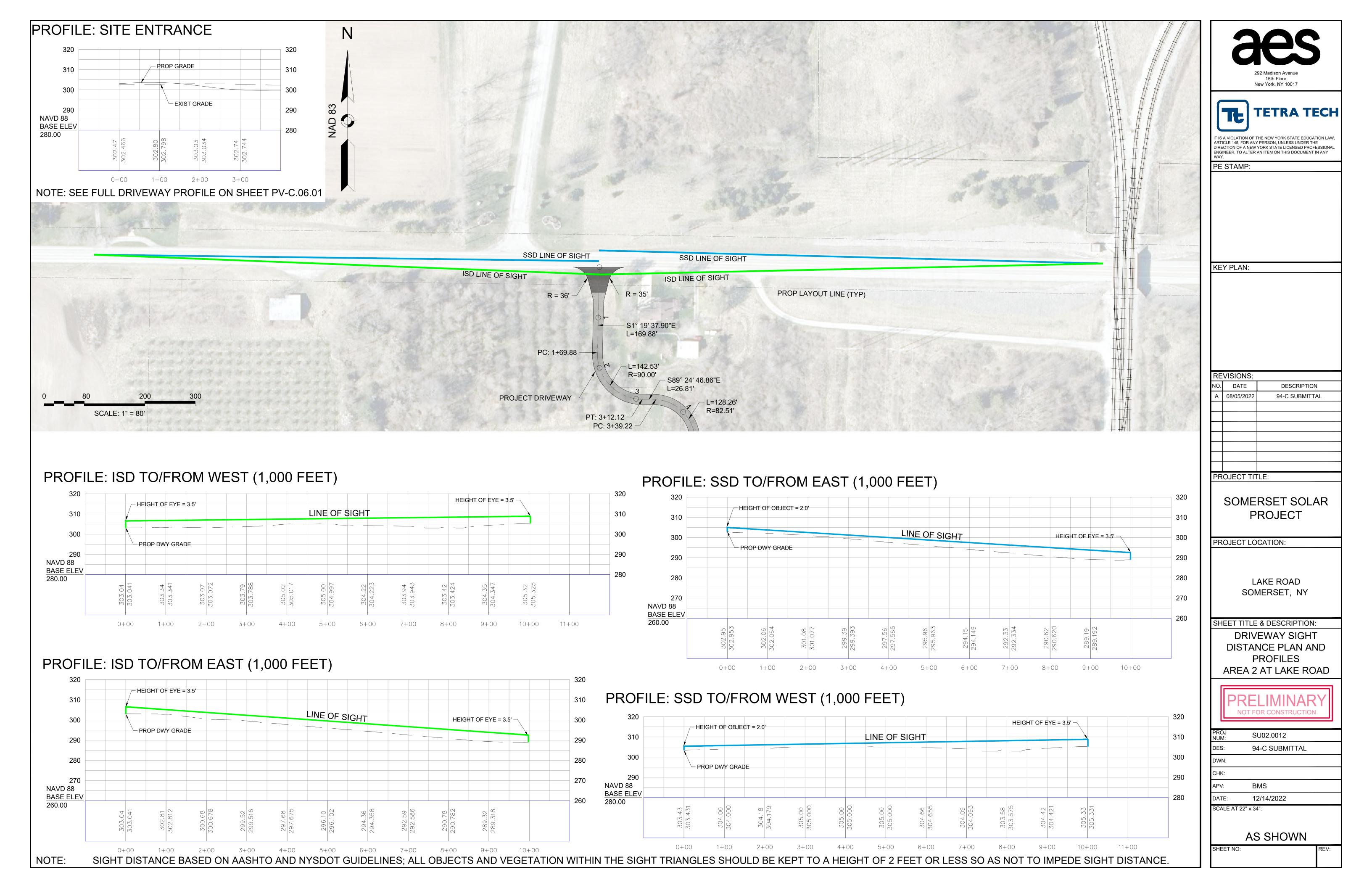
APPENDIX 16-A REVISED

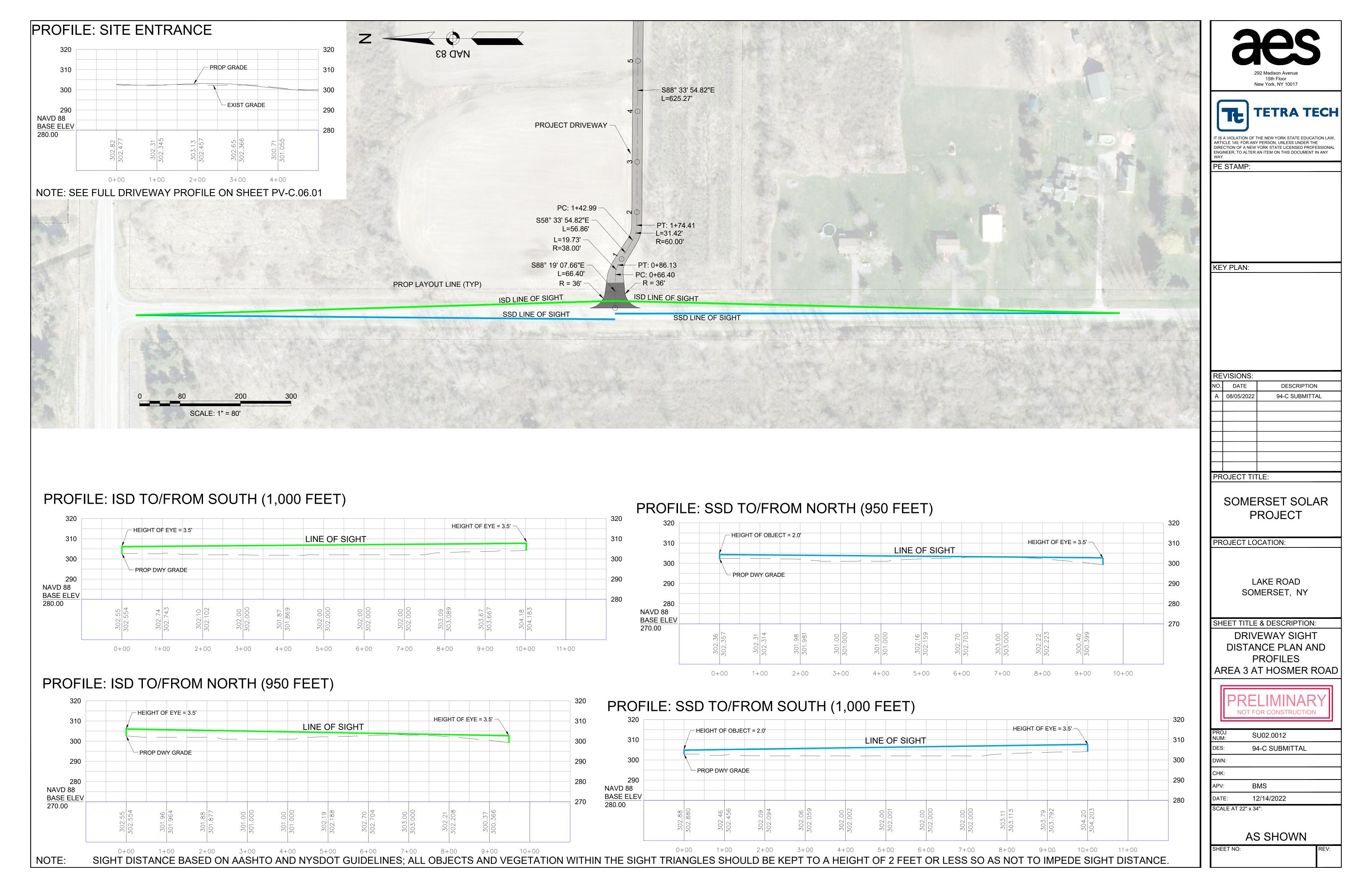
Transportation Data and Figures

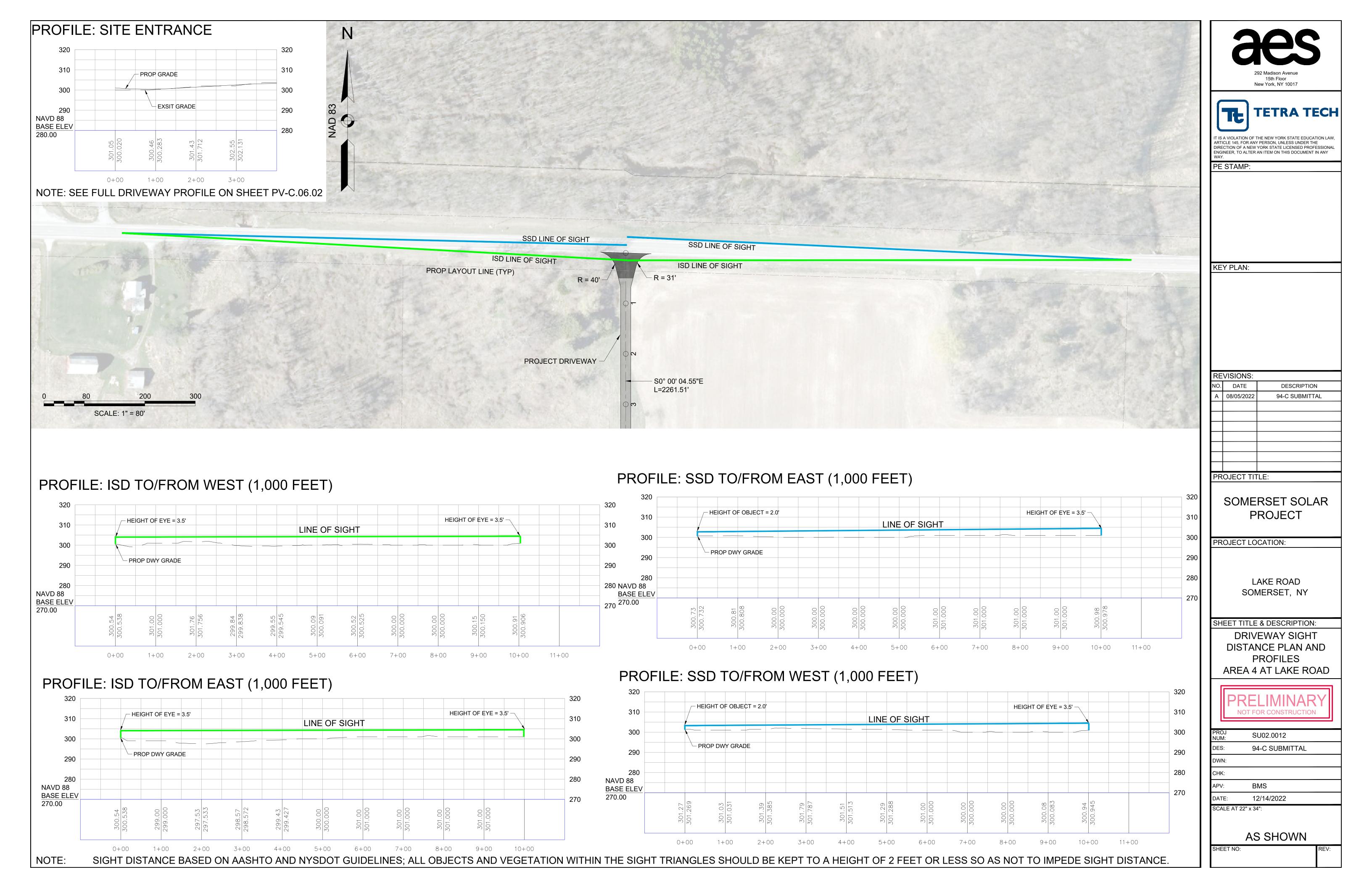


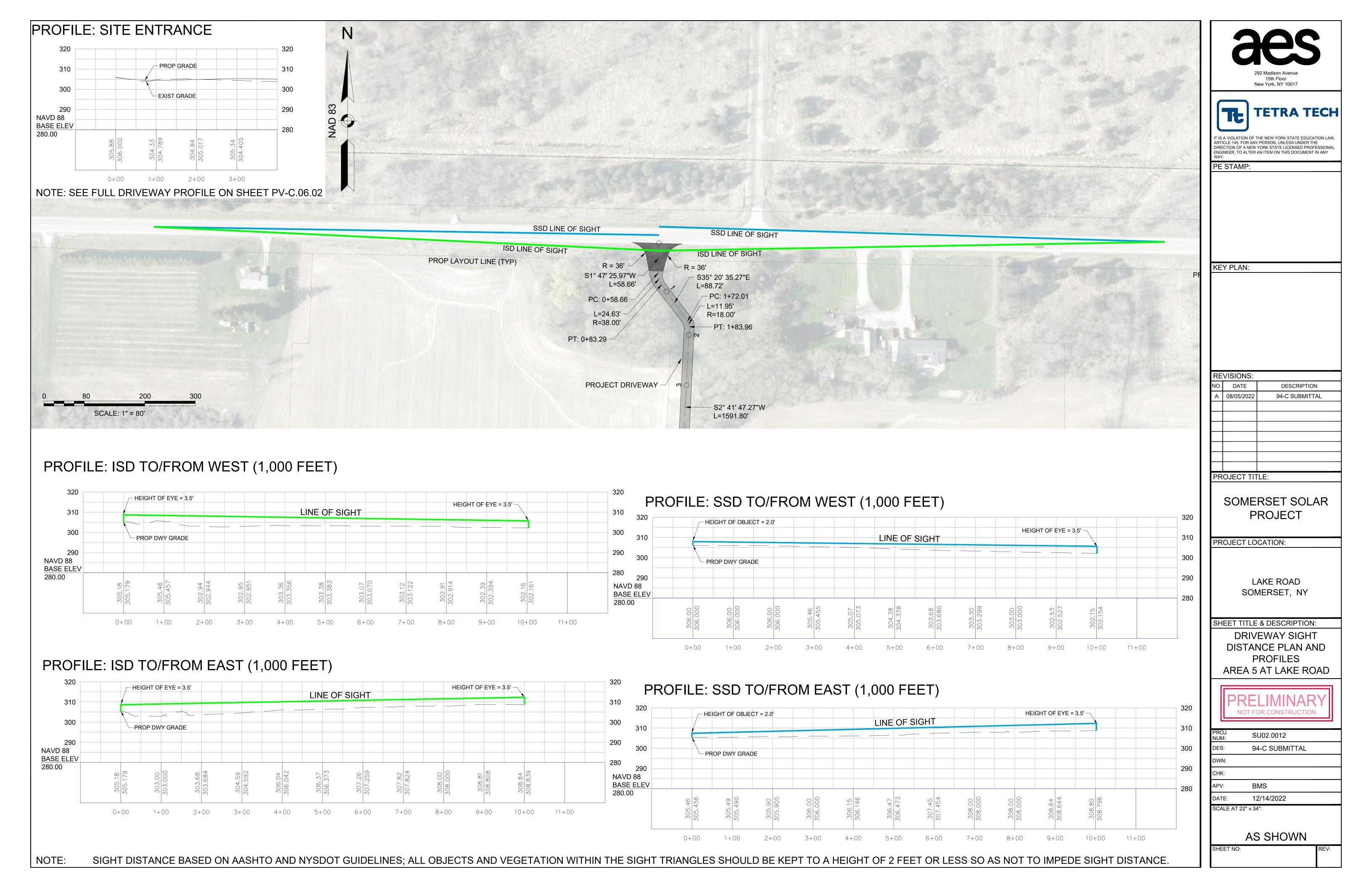


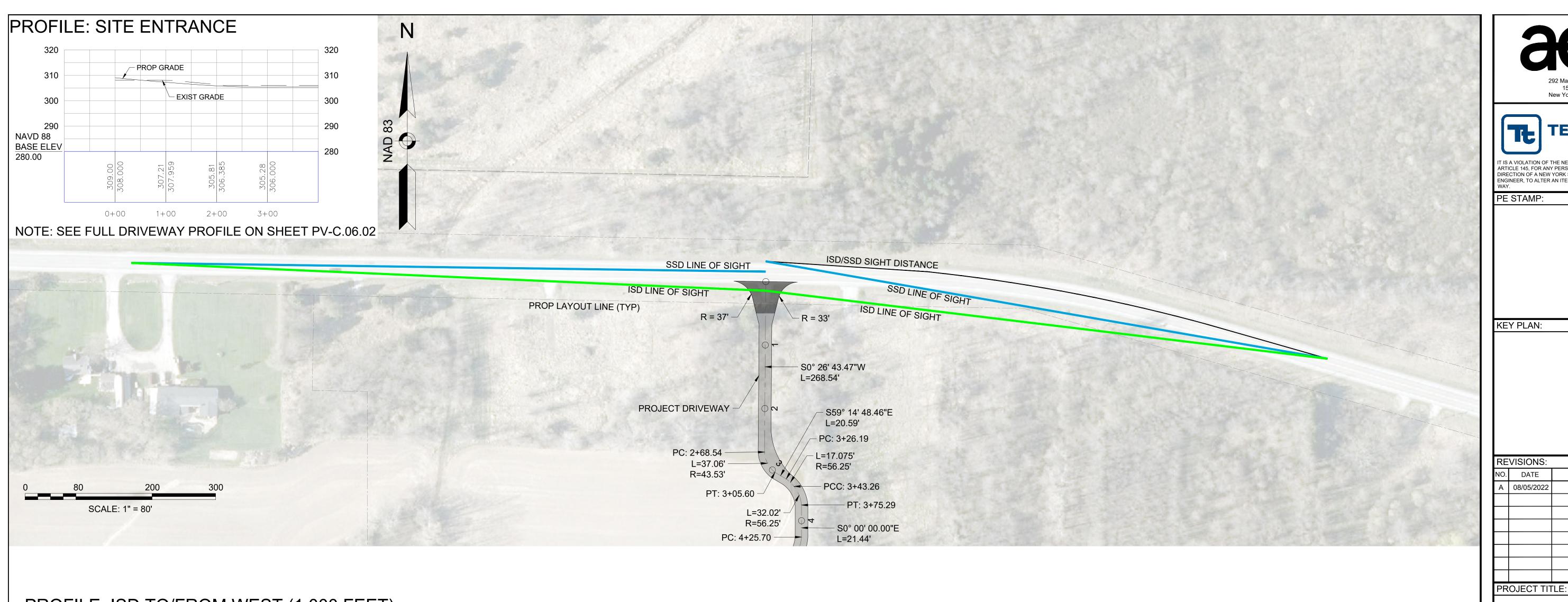




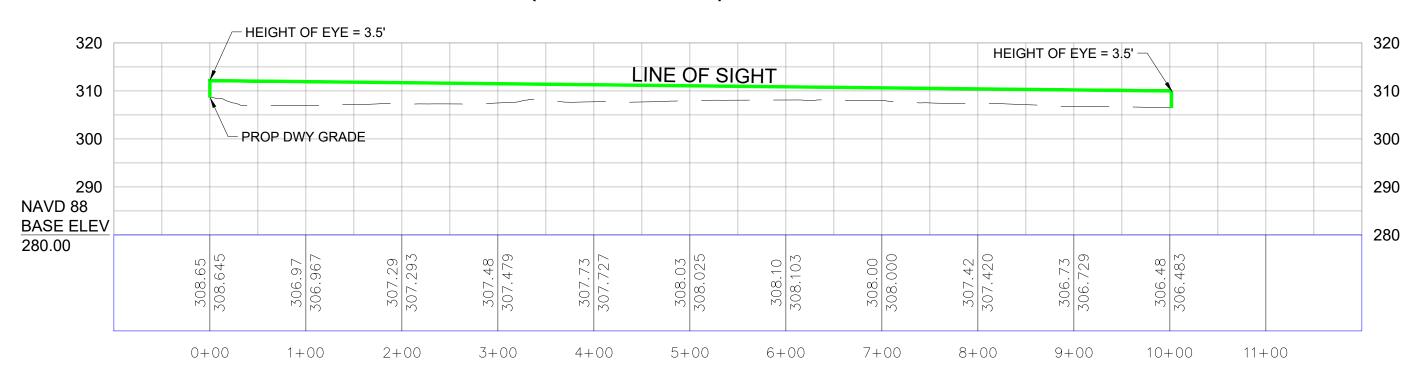




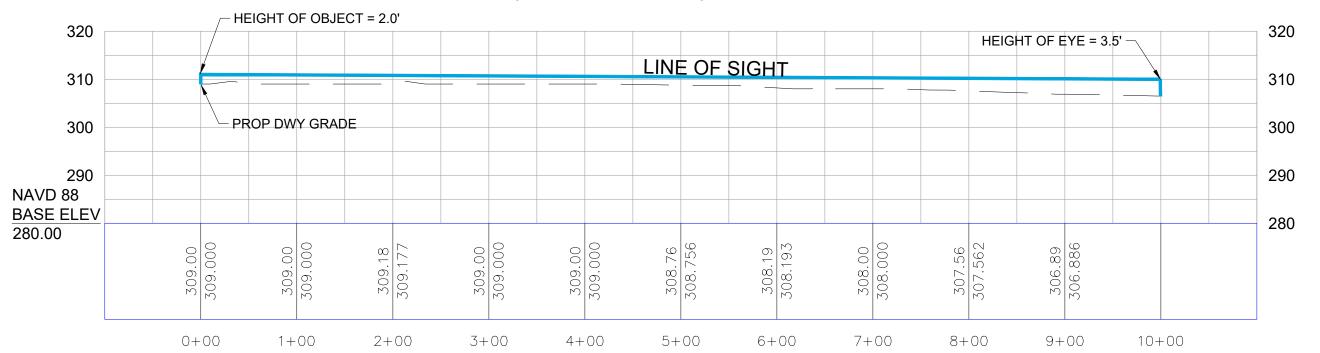




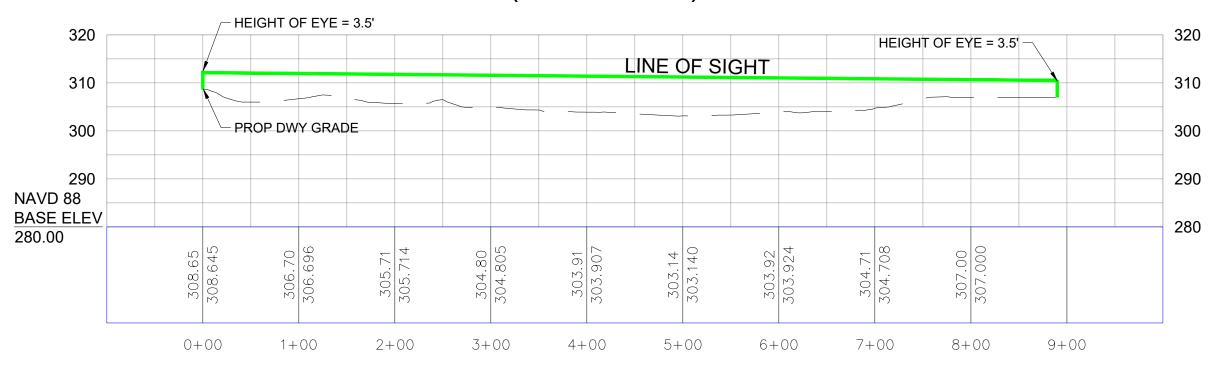
PROFILE: ISD TO/FROM WEST (1,000 FEET)



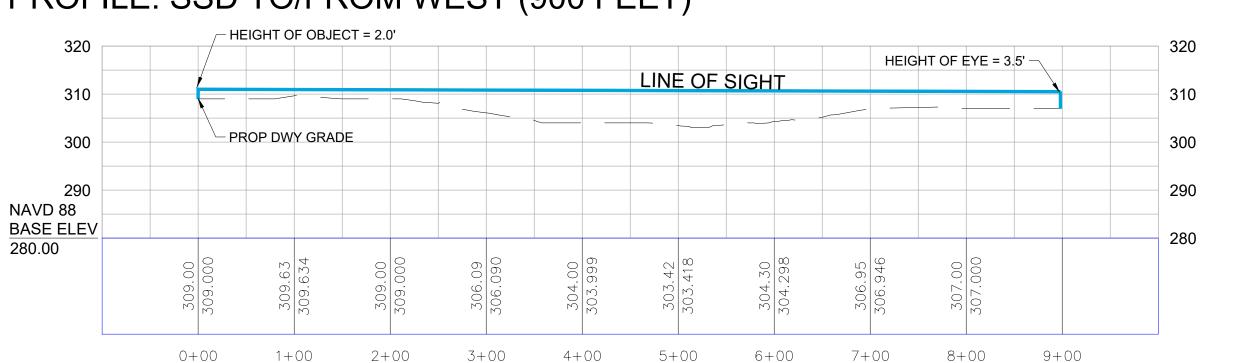




PROFILE: ISD TO/FROM EAST (>850 FEET)



PROFILE: SSD TO/FROM WEST (900 FEET)



New York, NY 10017



ARTICLE 145, FOR ANY PERSON, UNLESS UNDER THE DIRECTION OF A NEW YORK STATE LICENSED PROFESSIONAL ENGINEER, TO ALTER AN ITEM ON THIS DOCUMENT IN ANY

PE STAMP:

KEY PLAN:

REVISIONS: DESCRIPTION 94-C SUBMITTAL

> SOMERSET SOLAR **PROJECT**

PROJECT LOCATION:

LAKE ROAD SOMERSET, NY

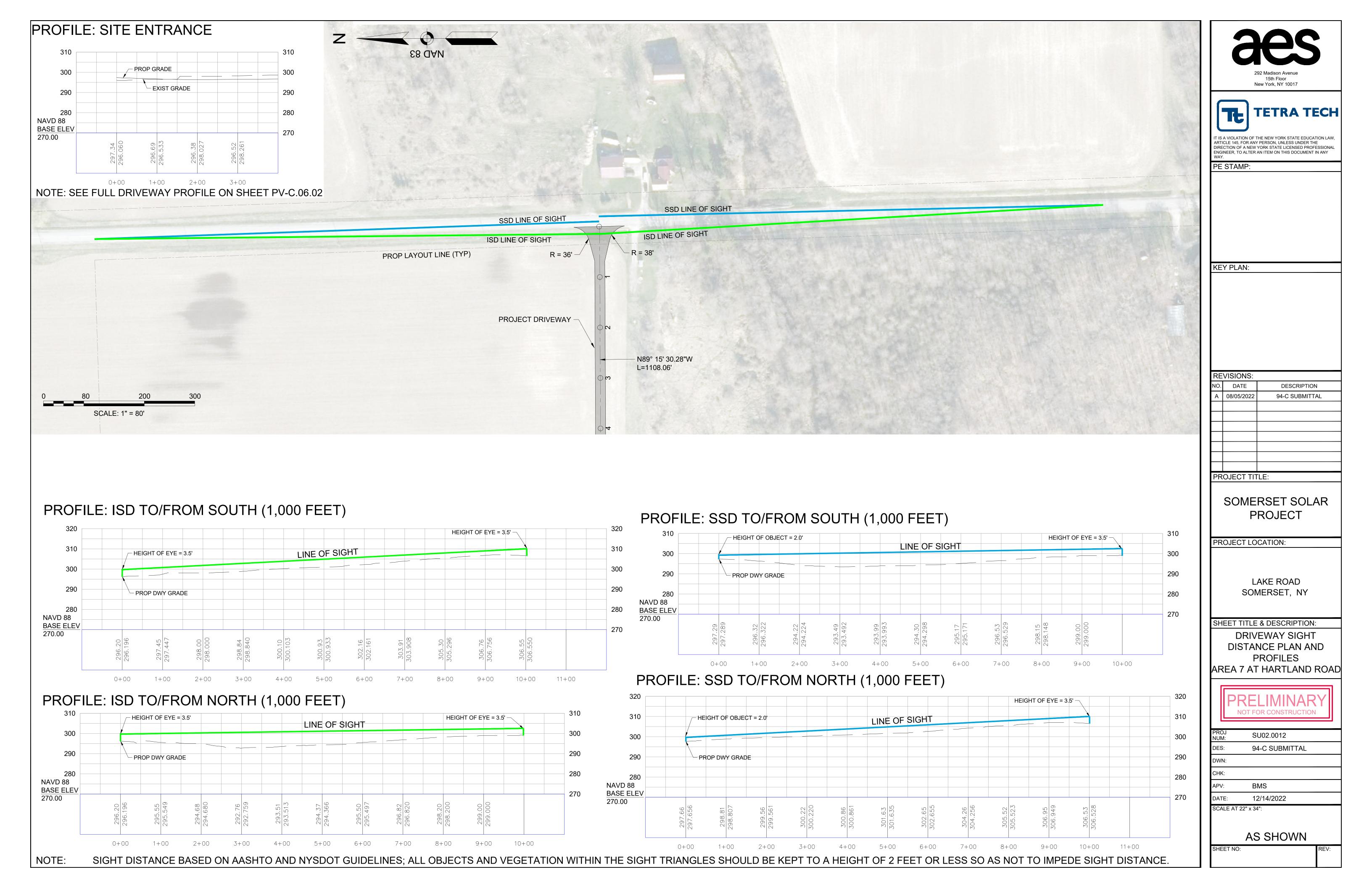
SHEET TITLE & DESCRIPTION: **DRIVEWAY SIGHT** DISTANCE PLAN AND **PROFILES** AREA 6 AT LAKE ROAD



SU02.0012 94-C SUBMITTAL BMS 12/14/2022 SCALE AT 22" x 34":

AS SHOWN SHEET NO:

NOTE: SIGHT DISTANCE BASED ON AASHTO AND NYSDOT GUIDELINES; ALL OBJECTS AND VEGETATION WITHIN THE SIGHT TRIANGLES SHOULD BE KEPT TO A HEIGHT OF 2 FEET OR LESS SO AS NOT TO IMPEDE SIGHT DISTANCE.





New York, NY 10017



IT IS A VIOLATION OF THE NEW YORK STATE EDUCATION LAW, ARTICLE 145, FOR ANY PERSON, UNLESS UNDER THE DIRECTION OF A NEW YORK STATE LICENSED PROFESSIONAL ENGINEER, TO ALTER AN ITEM ON THIS DOCUMENT IN ANY

PE STAMP:

KEY PLAN:

REVISIONS:

DESCRIPTION 94-C SUBMITTAL

PROJECT TITLE:

SOMERSET SOLAR **PROJECT**

PROJECT LOCATION:

LAKE ROAD SOMERSET, NY

SHEET TITLE & DESCRIPTION: **DRIVEWAY SIGHT**

DISTANCE PLAN AND **PROFILES** AREA 8 AT HARTLAND ROAD



SU02.0012 94-C SUBMITTAL

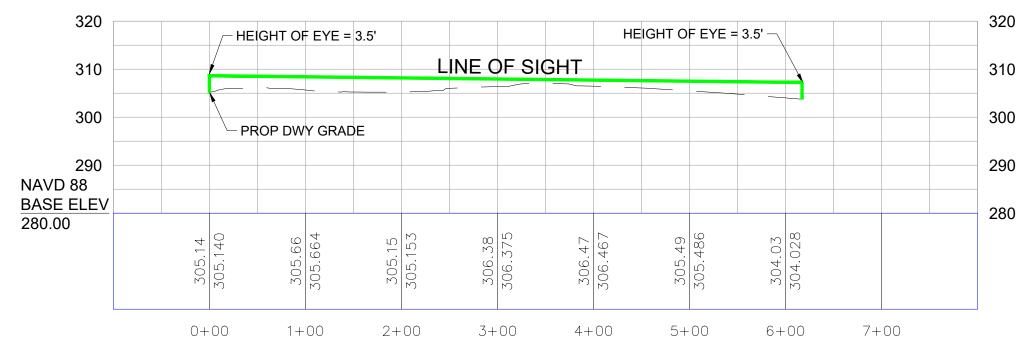
SCALE AT 22" x 34":

AS SHOWN

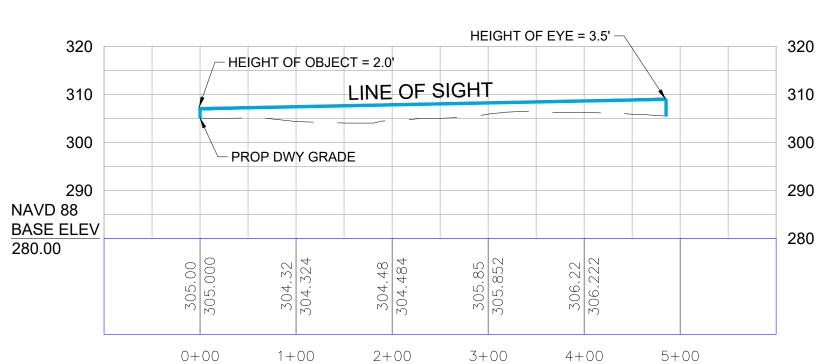
12/14/2022

PROFILE: ISD TO/FROM SOUTH (610 FEET)

NOTE:



PROFILE: SSD TO/FROM SOUTH (495 FEET)



SIGHT DISTANCE BASED ON AASHTO AND NYSDOT GUIDELINES; ALL OBJECTS AND VEGETATION WITHIN THE SIGHT TRIANGLES SHOULD BE KEPT TO A HEIGHT OF 2 FEET OR LESS SO AS NOT TO IMPEDE SIGHT DISTANCE. PROFILE TO AND FROM THE EAST (LT) IS UNAVAILABLE DUE TO LACK OF SURVEY AND AVAILABLE SIGHT DISTANCE BASED ON FIELD MEASUREMENTS, PHOTOS AND AERIAL IMAGERY

DESIGN CRITERIA

Exhibit 2-7 Design Criteria for Non-NHS Local Rural Roads

Design Speed	Travel Lane Width (ft.) Based on Design Year AADT ^{1,2}		Turn Lane (ft.)		Max. Percent Grade			Min. Stopping	Min. Radius Curve	
	AADT Under 400	Under 400- Over Minimum Desirable —		Lovel	Terrain	Mountainous	Sight Distance (ft.)	(ft.) e _{max} = 8%		
	400	2,000	2,0004			Level	Rolling	Mountainous		
20	9	10	11			8	11	16	97	70
25	9	10	11			7	11	15	133	113
30	9	10	11			7	11	15	175	167
35	9	10	11	40	Match Travel	7	10	14	220	233
40	9	10	11	10	Lane Width	7	10	13	271	314
45	10	11	11			7	9	12	327	409
50	10	11	11			6	8	10	387	521
55	11	11	11]		6	7	10	452	651

Shoulder Width (ft)¹

Notes:

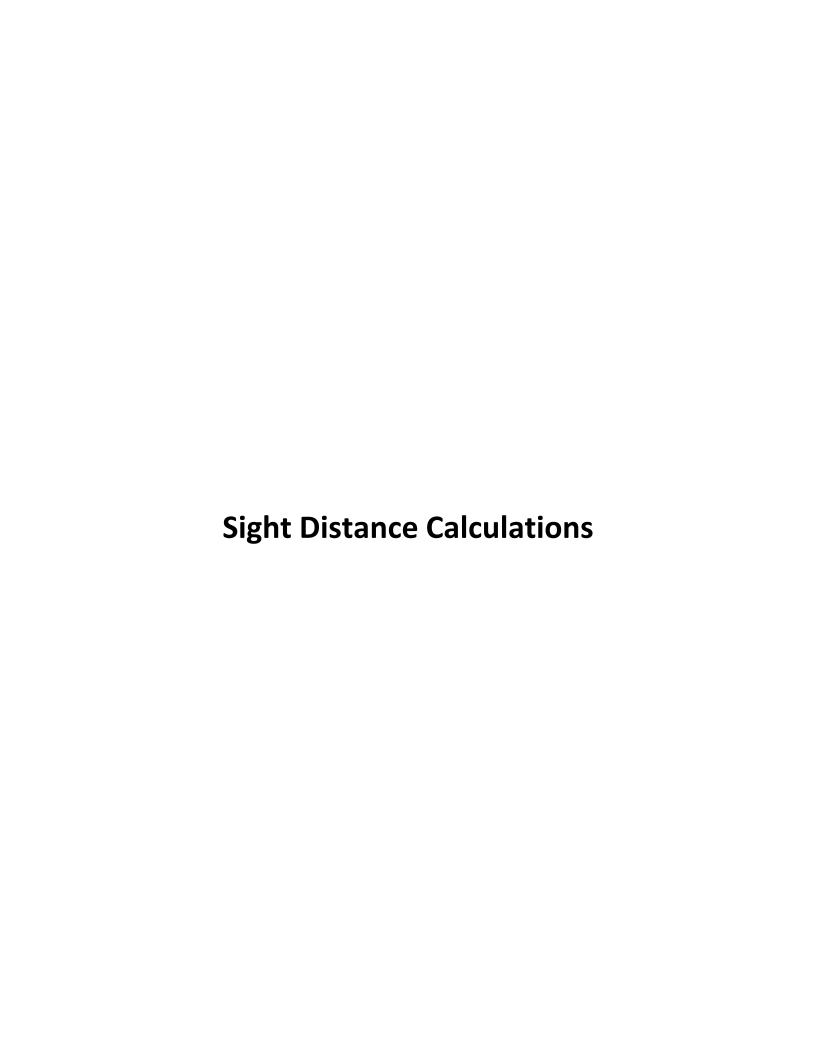
- For bridges, refer to the <u>NYSDOT Bridge Manual</u>, Section 2. Where the Bridge Manual only furnishes
 roadway width, subtract the lane width on this table from the roadway width to determine the shoulder
 width.
- 2. Minimum travel lane width is 10 ft. for routes designated as Access Highways and for routes within 1 mile of Qualifying Highways on the National Network (1982 STAA Highways).
- 3. For roads in mountainous terrain with a design volume under 3,000 AADT and a design speed of 40 mph or less, use 9 ft. lanes (except where Note 2 applies).
- 4. 12 ft. lanes should be considered where the crash rate is above the statewide rate for similar facilities and there is substantial truck traffic.
- 5. The minimum shoulder width is 4 ft. if roadside barrier is used on low-volume roads.
- 6. In uncurbed areas, a 4 ft. min. shoulder width is recommended where the route is a designated bicycle route or anticipated bicycle demand is high and cyclists will be accommodated on the shoulder. In curbed areas, a 4 ft. min. width shoulder, 5 ft. min. width bicycle lane or 13 ft. min. shared lane should be provided where there is high bicycling demand and/or a bicycle route is present and no parallel bicycle facility present. If neither the min. width shoulder nor lane can be provided, a justification is required for the nonstandard shoulder width. Refer to HDM §2.6.2.1 and Exhibit 2-1a.

All Speeds 2^{5,6} 4⁶ 6

APPENDIX 5C INTERSECTION SIGHT DISTANCE CHARTS

Table 5C-3 Design Intersection Sight Distance (in feet) - Case B1 - Left Turn From Stop

Design		Passenger (ingle-Unit T		Combination Truck		
speed	Lanes Crossed			Lanes Crossed			Lanes Crossed		
(mph)	1	2	3	1	2	3	1	2	3
15	170	180	190	210	225	245	255	270	285
20	225	240	250	280	300	325	340	360	380
25	280	295	315	350	375	405	425	450	475
30	335	355	375	420	450	485	510	540	570
35	390	415	440	490	525	565	595	630	665
40	445	475	500	560	600	645	680	720	760
45	500	530	565	630	675	725	765	810	855
50	555	590	625	700	750	805	850	900	950
55	610	650	690	770	825	885	930	990	1045
60	665	710	750	840	900	965	1015	1080	1140
65	720	765	815	910	975	1045	1100	1170	1235
70	775	825	875	980	1050	1125	1185	1260	1330



Location: Hartland Road at Proposed Solar Driveway

STOPPING SIGHT DISTANCE:

STOPPING SIGHT DISTANCE FROM SOUTH

Inputs

V=speed, mph V= 55 (Design Speed) G=percent of grade G= 0 (%) t=brake reaction time t= 2.5 a=deceleration rate, ft/sec2 11.2

Calculations

Brake Reaction Distance 1.47Vt 202 feet V²/30((a/32.2)+G) 289.9 feet Braking Distance Stopping Sight Distance = 1.47Vt + V²/30[(a/32.2)+G] 495 feet

STOPPING SIGHT DISTANCE FROM NORTH

Inputs

V=speed, mph V= 55 (Design Speed) G=percent of grade G= 0 (%) t=brake reaction time 2.5 t= a=deceleration rate, ft/sec2 a= 11.2

Calculations

Brake Reaction Distance 1.47Vt 202 feet V2/30((a/32.2)+G) 2<u>89.9</u> feet Braking Distance Stopping Sight Distance = 1.47Vt + V²/30[(a/32.2)+G] 495 feet

Source: A Policy on Geometric Design of Highways and Streets, 2018, Seventh Edition, prepared by AASHTO, p. 3-4 to 3-5.

INTERSECTION SIGHT DISTANCE:

INTERSECTION SIGHT DISTANCE - LEFT FROM MINOR APPROACH - TO THE SOUTH

Inputs

V= design speed, mph 55 (Design Speed) t=time gap for minor road vehicle to enter the major road 7.50 (choose value based on Table 1)

Calculations

Int. Sight Distance = 1.47Vt 610 feet

Table 1 - Time Gap Factors								
Design Vehicle	Time Gap ¹ , t (sec) for Grades =3%</th <th>Grade of Minor Approach</th> <th>Number of Additional Lanes to Cross</th> <th>Adjusted Time Gap, t (sec)</th>	Grade of Minor Approach	Number of Additional Lanes to Cross	Adjusted Time Gap, t (sec)				
passenger car	7.5	0%	0	7.50				
single-unit truck	9.5	0%	0	9.50				
combination truck	11.5	0%	0	11.50				

INTERSECTION SIGHT DISTANCE - LEFT FROM MINOR APPROACH - TO THE NORTH

Inputs

V= design speed, mph 55 (Design Speed)

t=time gap for minor road vehicle to enter the major road 7.50 (choose value based on Table 1)

Calculations

610 feet 1.47Vt Int. Sight Distance =

Design Vehicle	Time Gap ¹ , t (sec) for Grades =3%</th <th>Grade of Minor Approach</th> <th>Additional Lanes to Cross</th> <th>Adjusted Time Gap, t (sec)</th>	Grade of Minor Approach	Additional Lanes to Cross	Adjusted Time Gap, t (sec)
passenger car	7.5	0%	0	7.50
single-unit truck	9.5	0%	0	9.50
combination truck	11.5	0%	0	11.50

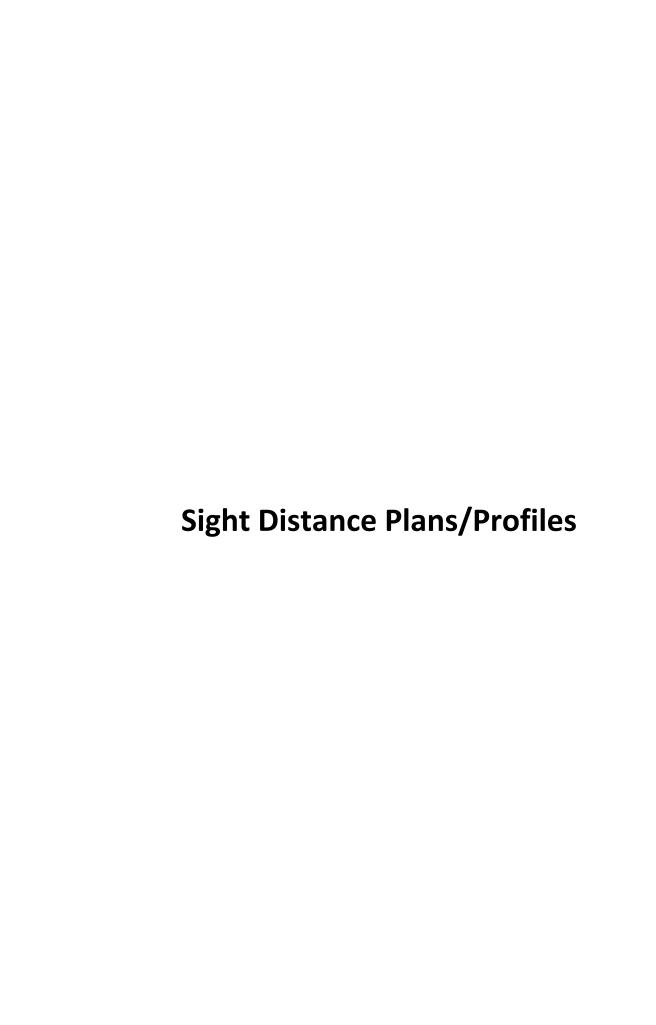
Notes:

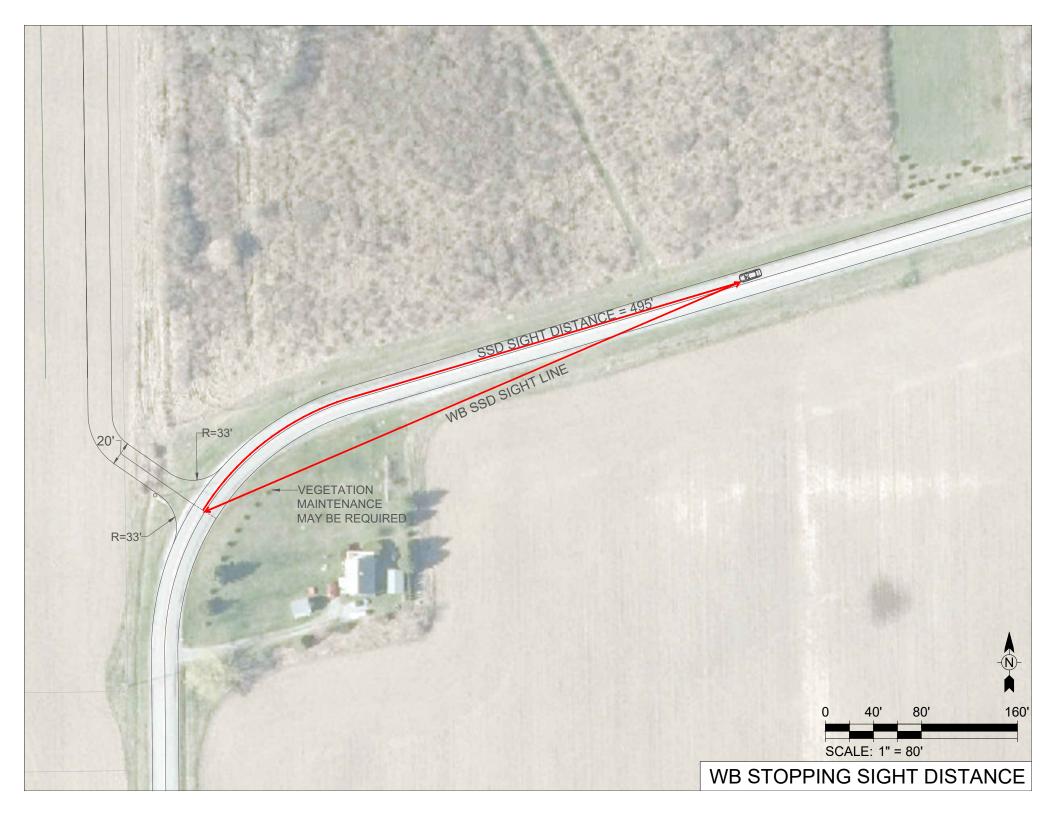
1. Time Gap values are applicable for major roads with grades 3 percent or less and no median and a minor street approach with a grade of 3 percent or less. Otherwise, the table values should be adju:

^{*}If the minor street has an upward grade of more than 3 percent then add 0.2 sec. to t for each percent grade (including the first 3 percent).

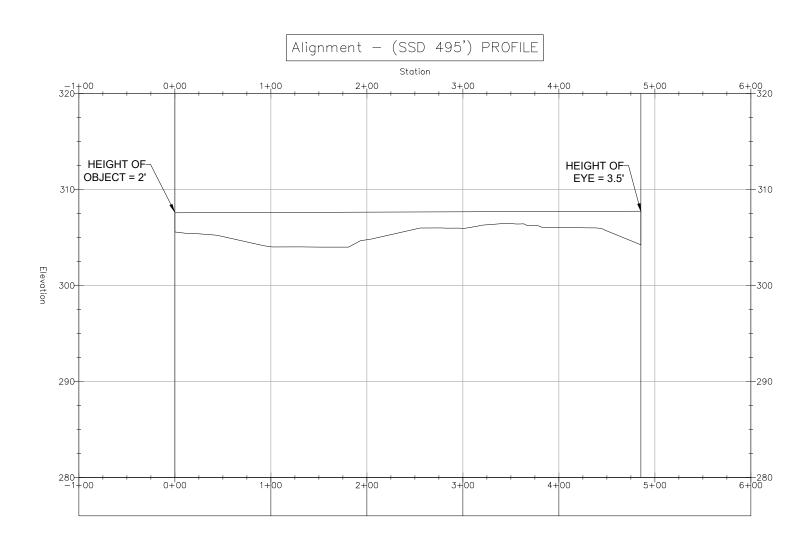
**Increase t by 0.5 seconds (for passenger cars) or 0.7 seconds (for trucks) for every additional lane from the left, in excess of one, to be crossed by the turning vehicle.

**If the major approach is a divided highway with a median not wide enough to store the design vehicle, then the median width should be converted to equivalent lanes



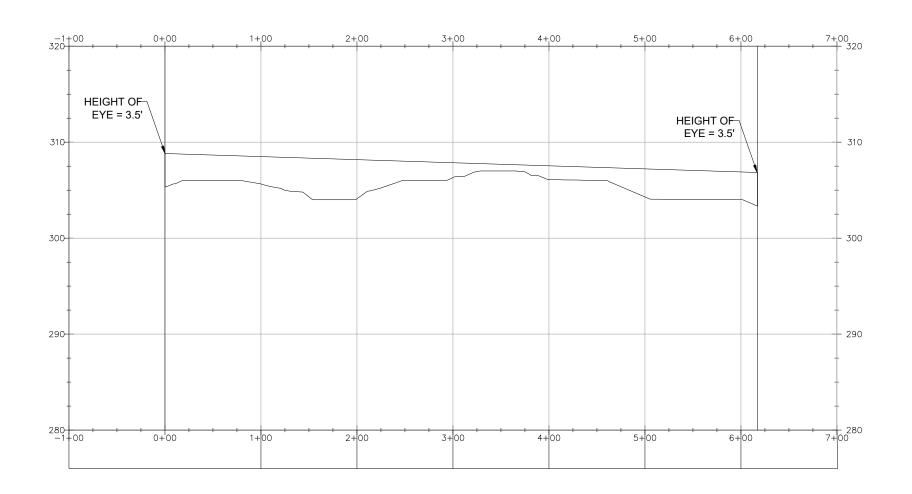






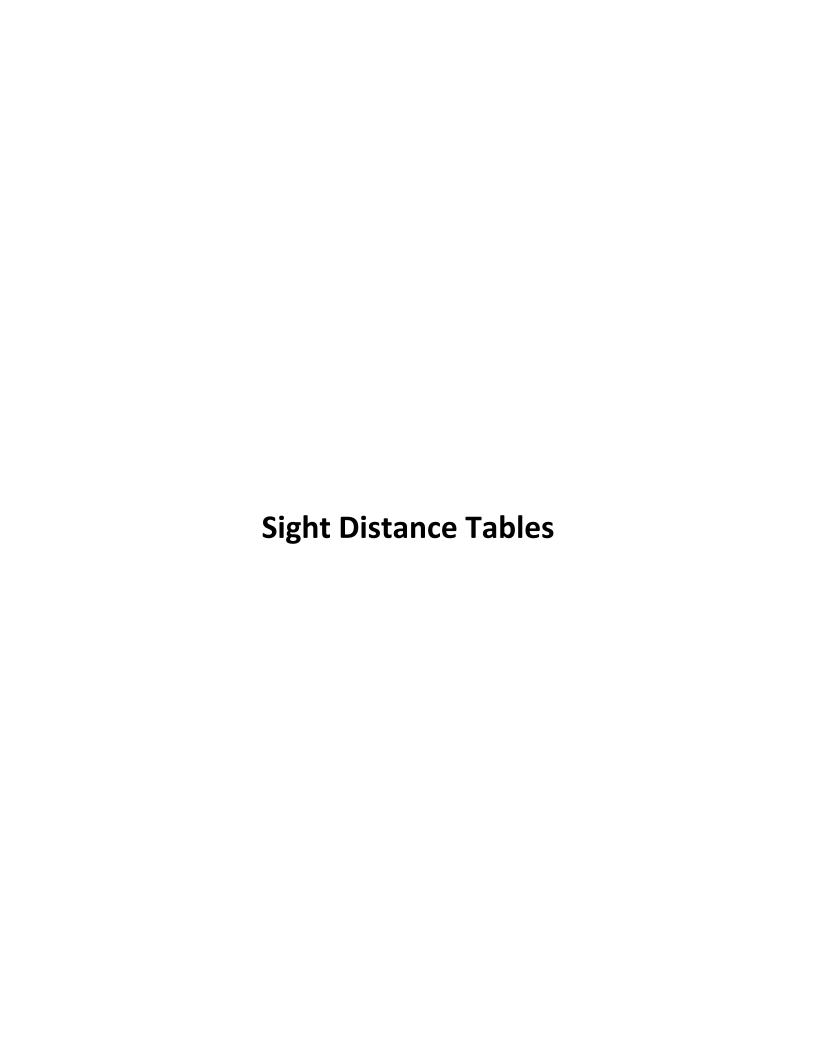


Alignment - (ISD 610') PROFILE



PROFILE TO AND FROM EAST UNAVAILABLE DUE TO LACK OF SURVEY

EB INTERSECTION SIGHT DISTANCE PROFILES



AASHTO Sight Distance

Scenario 1: Stopping Sight Distance on Level Roadways

Table 3-1. Stopping Sight Distance on Level Roadways

	U.S. Customary					Metric					
Design	Brake	Braking	Stopp	ing	Design	Brake	Braking	Stopping			
Speed	Reaction	Distance	Sight Dis	stance	Speed	Reaction	Distance	Sight Dis	stance		
(mph)	Distance	on Level	Calculated	Design	(km/h)	Distance	on Level	Calculated	Design		
	(ft)	(ft)	(ft)	(ft)		(m)	(m)	(m)	(m)		
15	55.1	21.6	76.7	80	20	13.9	4.6	18.5	20		
20	73.5	38.4	111.9	115	30	20.9	10.3	31.2	35		
25	91.9	60.0	151.9	155	40	27.8	18.4	46.2	50		
30	110.3	86.4	196.7	200	50	34.8	28.7	63.5	65		
35	128.6	117.6	246.2	250	60	41.7	41.3	83.0	85		
40	147.0	153.6	300.6	305	70	48.7	56.2	104.9	105		
45	165.4	194.4	359.8	360	80	55.6	73.4	129.0	130		
50	183.8	240.0	423.8	425	90	62.6	92.9	155.5	160		
55	202.1	290.3	492.4	495	100	69.5	114.7	184.2	185		
60	220.5	345.5	566.0	570	110	76.5	138.8	215.3	220		
65	238.9	405.5	644.4	645	120	83.4	165.2	248.6	250		
70	257.3	470.3	727.6	730	130	90.4	193.8	284.2	285		
75	275.6	539.9	815.5	820	140	97.3	224.8	322.1	325		
80	294.0	614.3	908.3	910							
85	313.5	693.5	1007.0	1010	1						

Note: Brake reaction distance predicated on a time of 2.5 s; deceleration rate of 11.2 ft/s 2 [3.4 m/s 2] used to determine calculated sight distance.

Scenario 2: Stopping Sight Distance on Grades

Table 3-2. Stopping Sight Distance on Grades

U.S. Customary										
Design	Stopping Sight Distance (ft)									
Speed	Do	wngrad	des	J	Upgrades					
(mph)	3%	6%	9%	3%	6%	9%				
15	80	82	85	75	74	73				
20	116	120	126	109	107	104				
25	158	165	173	147	143	140				
30	205	215	227	200	184	179				
35	257	271	287	237	229	222				
40	315	333	354	289	278	269				
45	378	400	427	344	331	320				
50	446	474	507	405	388	375				
55	520	553	593	469	450	433				
60	598	638	686	538	515	495				
65	682	728	785	612	584	561				
70	771	825	891	690	658	631				
75	866	927	1003	772	736	704				
80	965	1035	1121	859	817	782				
85	1070	1149	1246	949	902	862				

Design Speed (km/h) Stopping Sight Distance (respective) Downgrades Upgrad (km/h) 3% 6% 9% 3% 6% 20 20 20 19 18 30 32 35 35 31 30 40 50 50 53 45 44 50 66 70 74 61 59	Metric								
(km/h) 3% 6% 9% 3% 6% 20 20 20 20 19 18 30 32 35 35 31 30 40 50 50 53 45 44 50 66 70 74 61 59	1)	Design							
20 20 20 20 19 18 30 32 35 35 31 30 40 50 50 53 45 44 50 66 70 74 61 59	es	pgrade	U	des					
30 32 35 35 31 30 40 50 50 53 45 44 50 66 70 74 61 59	9%	6%	3%	9%	6%	3%	(km/h)		
40 50 50 53 45 44 50 66 70 74 61 59	18	18	19	20	20	20	20		
50 66 70 74 61 59	29	30	31	35	35	32	30		
	43	44	45	53	50	50	40		
(0 07 00 07 00 77	58	59	61	74	70	66	50		
60 87 92 97 80 77	75	77	80	97	92	87	60		
70 110 116 124 100 97	93	97	100	124	116	110	70		
80 136 144 154 123 118	114	118	123	154	144	136	80		
90 164 174 187 148 141	136	141	148	187	174	164	90		
100 194 207 223 174 167	160	167	174	223	207	194	100		
110 227 243 262 203 194	186	194	203	262	243	227	110		
120 263 281 304 234 223	214	223	234	304	281	263	120		
130 302 323 350 267 254	243	254	267	350	323	302	130		
140 341 367 398 302 287	274	287	302	398	367	341	140		

Scenario 3: Intersection Sight Distance – Left Turn from Stop

Table 9-7. Design Intersection Sight Distance—Case B1, Left Turn from Stop

	U.S. Customary							
Design Speed	Stopping Sight	Intersection Sight Distance for Passenger Cars						
(mph)	Distance (ft)	Calculated (ft)	Design (ft)					
15	80	165.4	170					
20	115	220.5	225					
25	155	275.6	280					
30	200	330.8	335					
35	250	385.9	390					
40	305	441.0	445					
45	360	496.1	500					
50	425	551.3	555					
55	495	606.4	610					
60	570	661.5	665					
65	645	716.6	720					
70	730	771.8	775					
75	820	826.9	830					
80	910	882.0	885					

Metric							
Design Speed	Stopping Sight Distance	Intersection Sight Distance for Passenger Cars					
(km/h)	(m)	Calculated (m)	Design (m)				
20	20	41.7	45				
30	35	62.6	65				
40	50	83.4	85				
50	65	104.3	105				
60	85	125.1	130				
70	105	146.0	150				
80	130	166.8	170				
90	160	187.7	190				
100	185	208.5	210				
110	220	229.4	230				
120	250	250.2	255				
130	285	271.1	275				

Note: Intersection sight distance shown is for a stopped passenger car to turn left onto a two-lane highway with no median and grades 3 percent or less. For other conditions, the time gap should be adjusted and the sight distance recalculated.

Scenario 4: Intersection Sight Distance – Right Turn from Stop

Table 9-9. Design Intersection Sight Distance—Case B2, Right Turn from Stop

	U.S. Customary							
Design Speed (mph)	Stopping Sight Distance	Intersection Sight Distance for Passenger Cars						
	(ft)	Calculated	Design					
		(ft)	(ft)					
15	80	143.3	145					
20	115	191.1	195					
25	155	238.9	240					
30	200	286.7	290					
35	250	334.4	335					
40	305	382.2	385					
45	360	430.0	430					
50	425	477.8	480					
55	495	525.5	530					
60	570	573.3	575					
65	645	621.1	625					
70	730	668.9	670					
75	820	716.6	720					
80	910	764.4	765					

Metric							
Design Speed (km/h)	Stopping Sight Distance	Intersection Sight Distance for Passenger Cars					
	(m)	Calculated (m)	Design (m)				
20	20	36.1	40				
30	35	54.2	55				
40	50	72.3	75				
50	65	90.4	95				
60	85	108.4	110				
70	105	126.5	130				
80	130	144.6	145				
90	160	162.6	165				
100	185	180.7	185				
110	220	198.8	200				
120	250	216.8	220				
130	285	234.9	235				

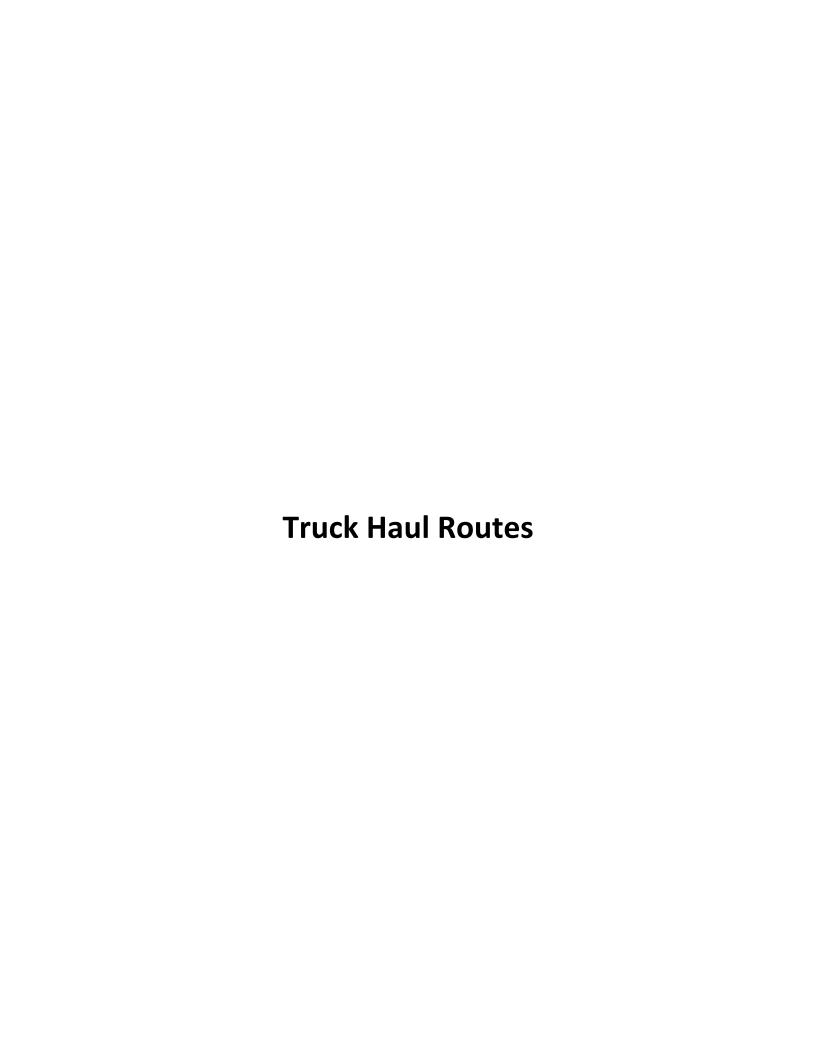
Note: Intersection sight distance shown is for a stopped passenger car to turn right onto or to cross a two-lane roadway with no median and with grades of 3 percent or less. For other conditions, the time gap should be adjusted and the sight distance recalculated.

Scenario 5: Intersection Sight Distance – Left Turn from Major Road

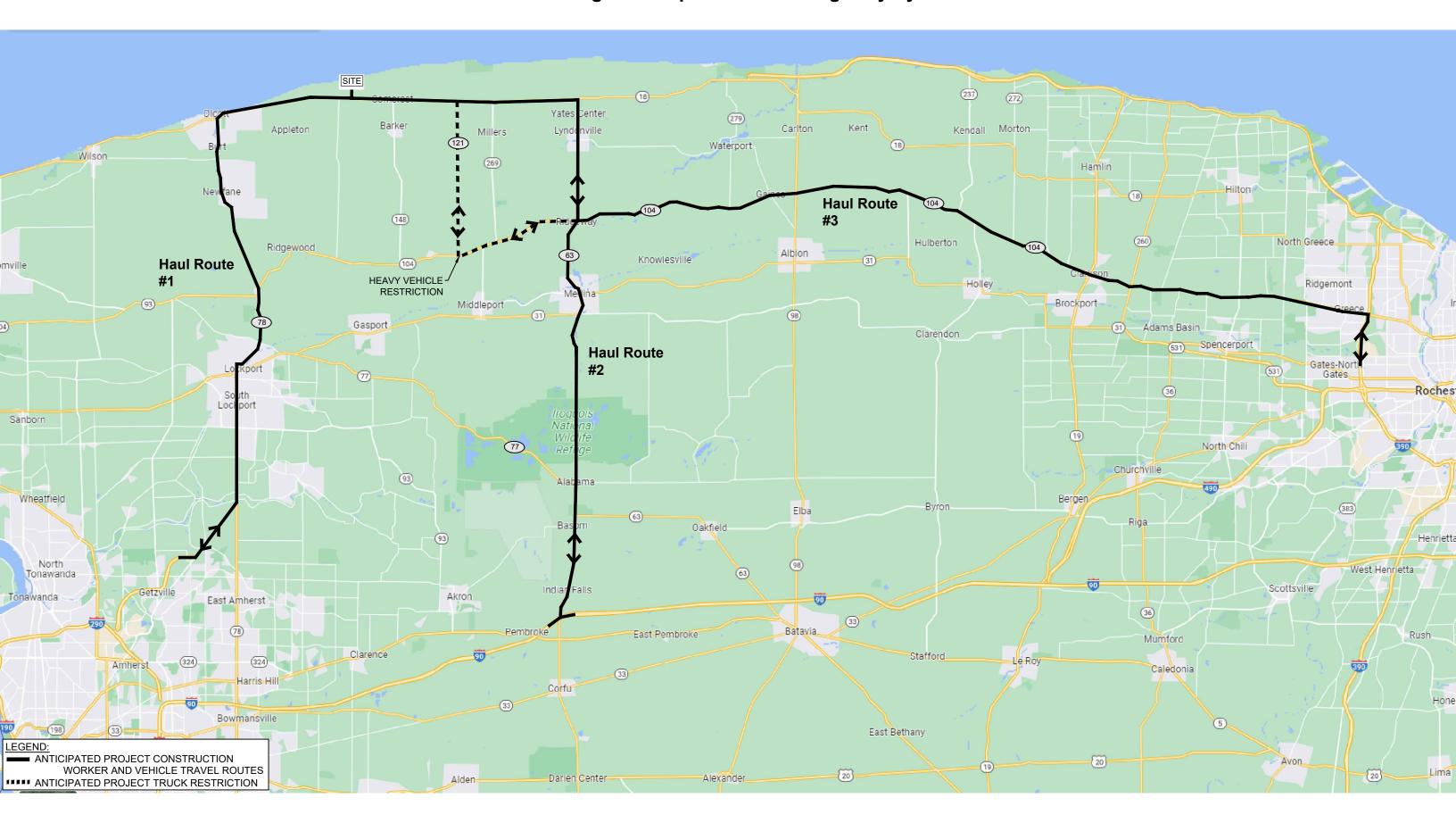
Table 9-17. Intersection Sight Distance—Case F, Left Turn from the Major Road

	U.S. Customary					Metric					
Design	Stopping	Interse Sight D			Design	Stopping	Intersection Sight Distance				
Speed	Sight Distance	Passenger Cars			Speed	Sight Distance	Passeng	jer Cars			
(mph)	(ft)	Calculated (ft)	Design (ft)		(km/h)	(m)	Calculated (m)	Design (m)			
15	80	121.3	125		20	20	30.6	35			
20	115	161.7	165		30	35	45.9	50			
25	155	202.1	205		40	50	61.2	65			
30	200	242.6	245		50	65	76.5	80			
35	250	283.0	285		60	85	91.7	95			
40	305	323.4	325		70	105	107.0	110			
45	360	363.8	365		80	130	122.3	125			
50	425	404.3	405		90	160	137.6	140			
55	495	444.7	445		100	185	152.9	155			
60	570	485.1	490		110	220	168.2	170			
65	645	525.5	530		120	250	183.5	185			
70	730	566.0	570		130	285	198.8	200			
75	820	606.4	610	Ι.							
80	910	646.8	650								

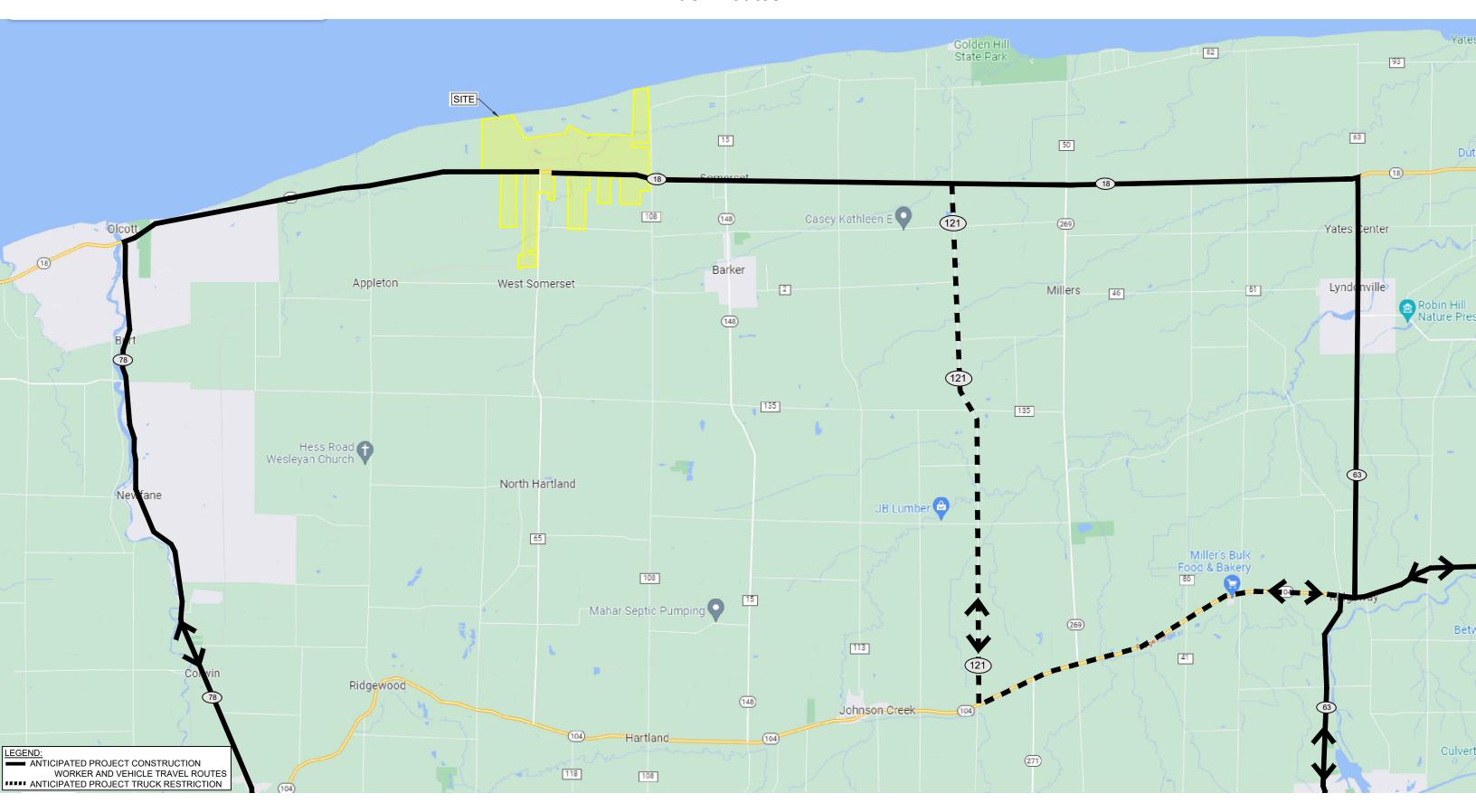
Note: Intersection sight distance shown is for a passenger car making a left turn from an undivided roadway. For other conditions and design vehicles, the time gap should be adjusted and the sight distance recalculated.

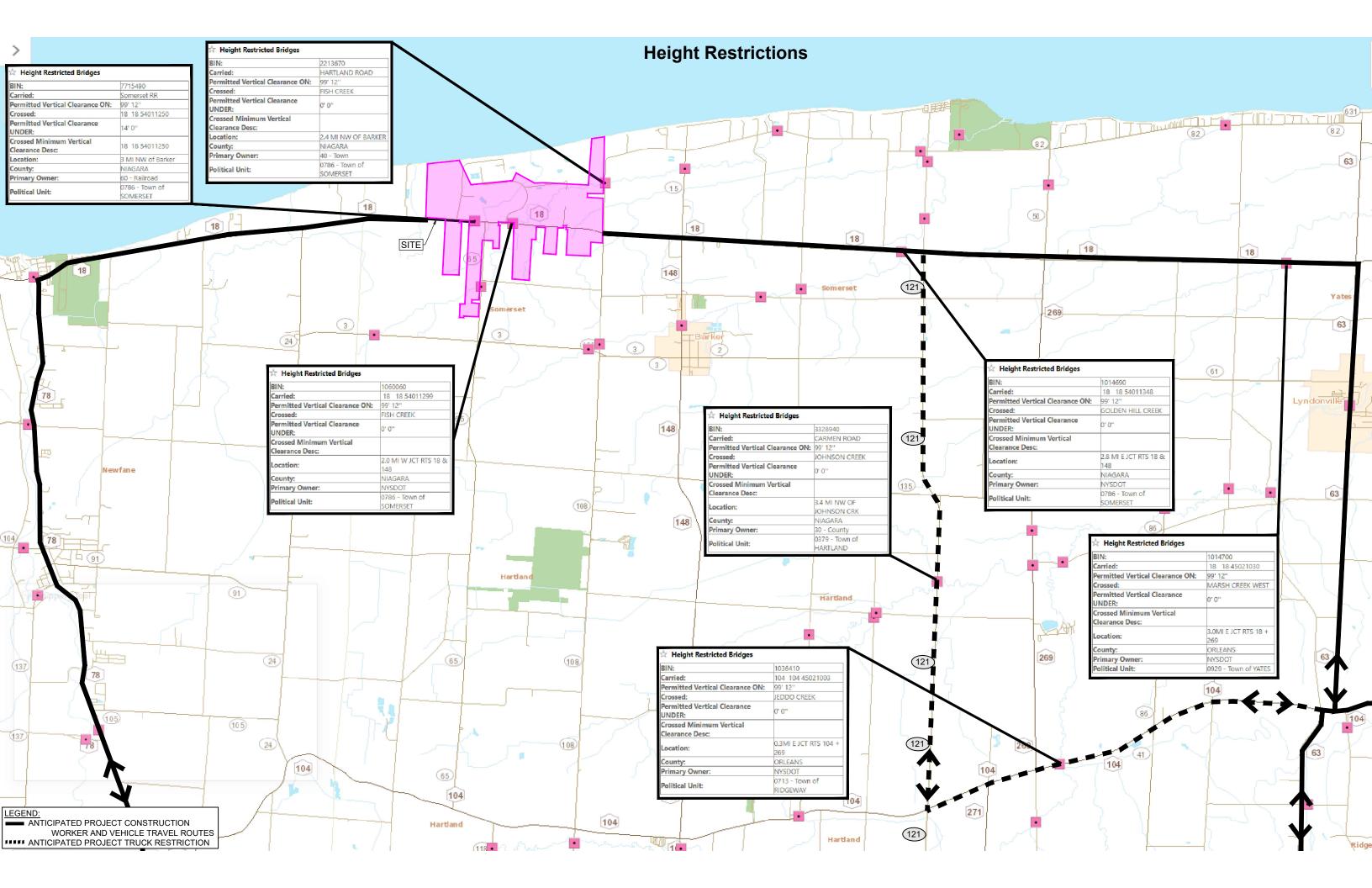


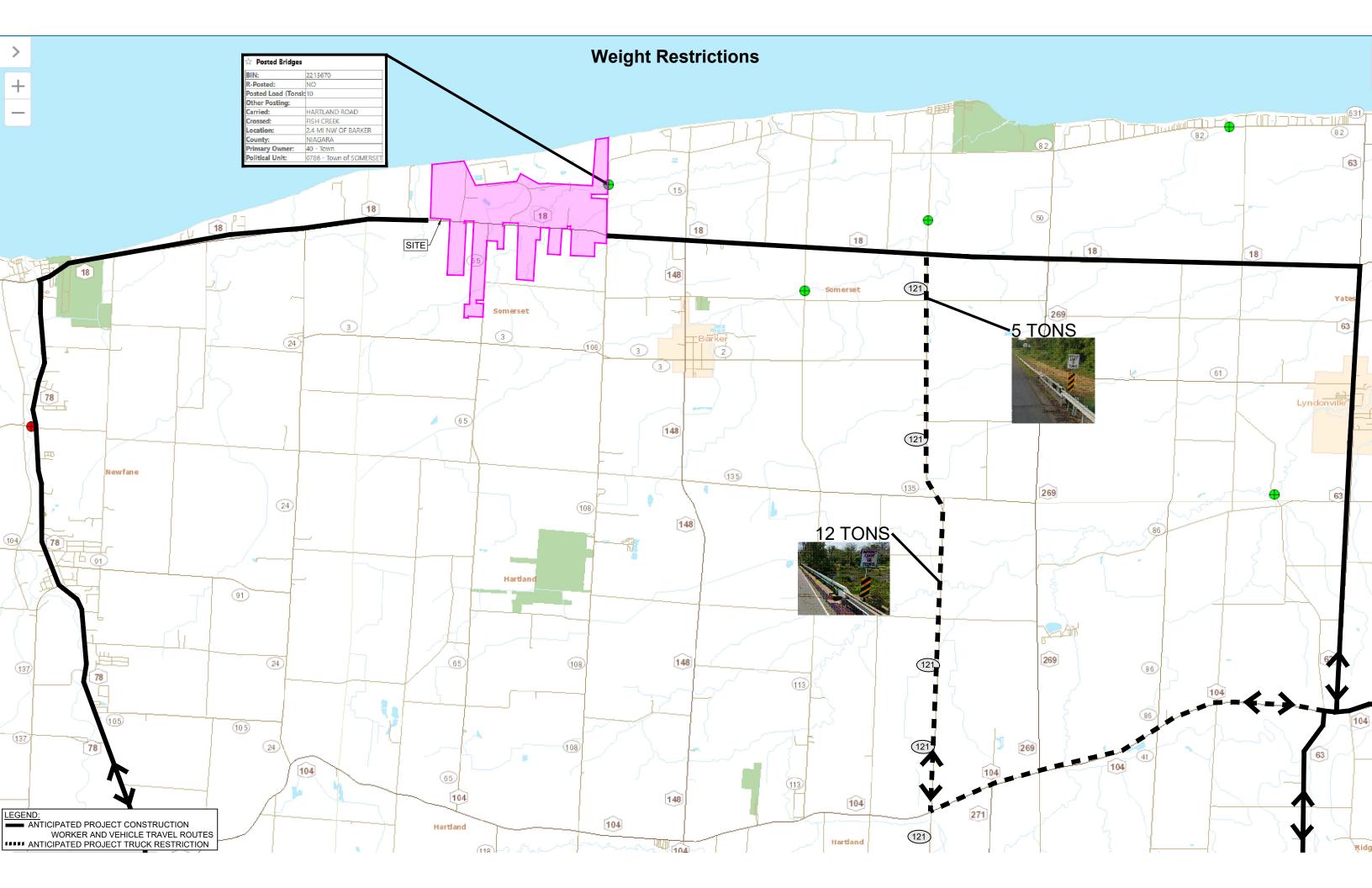
Regional Map to Interstate Highway Systems



Truck Routes







Roadway Surface Ratings

